# **110 Jervois Quay**

## Civil Engineering Resource Consent Design Report

# **MFC Development LP**

Reference: 520408 Revision: D 2022-03-03





# **Document control record**

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# **Executive Summary**

The key conclusions from each section of this civil engineering report related to the resource consent are:

- There is capacity in existing 3-waters services to support the proposed building development;
- There is a projected risk of flooding from climate change which has been factored into the proposed building floor levels. Modelling also confirms the impact of the building on the surrounding area is negligible;
- There are existing gas and HV main adjacent the proposed development in Wakefield Street which be protected during the works;
- There is a large stormwater culvert that runs underneath the proposed building at the east of the site.
   Ongoing discussions are being progressed with Wellington Water regarding treatment of this culvert;
- There is a large wastewater buffer tank on the west of the site that is affected by the works. Ongoing discussions are being progressed with Wellington Water regarding treatment of this tank;

# 1 Introduction

# 1.1 **Project Description**

The applicant is developing a new Green Star rated office building at 110 Jervois Quay, on the site of the existing carpark. The building will consist of a ground floor, seven office levels and a rooftop plant room and plant deck. The development will also include external landscaping, enhancing the site and its connectivity to the waterfront.

The Figure below shows the proposed development layout.



Figure 1: Proposed Site Plan

# 1.2 Site description

The site is located at 110 Jervois Quay and is bounded by the Michael Fowler Centre (MFC) building to the west, Wakefield Street to the south and south-west, and Jervois Quay to the north and east. The surrounding area generally comprises commercial and residential apartment buildings.

The site currently accommodates the temporary Royal New Zealand Ballet building, with a mixture of open pedestrian space and asphalt car parking on its western side. On its eastern side is a small park comprising grassed areas, planted garden beds, and trees. The site is generally level.

Vehicle access to the site is from Wakefield Street; there is also a significant pedestrian link across Wakefield St through the site from Cuba Street across Jervois Quay to the waterfront, with a traffic light operated pedestrian crossing controlling movements.

### 1.3 Design Standards

The design for all stages of the project will be undertaken in accordance with national and regional standards. These include but not limited to:

- NZ Building Code
- Wellington Water Regional Standards for Water Services (December 2021)
- Wellington Water Regional Specification for Water Services (December 2021)
- SNZ PAS 4509:2008 New Zealand Fire Service Firefighting Water Supplies Code of Practice
- Erosion and Sediment Control Guide for Land Disturbing Activities in the Wellington Region (Feb 2021)

# 1.4 Available Information

The underground services base plan has been produced utilising information available to Aurecon at the time of writing this report.

Features and service positions shown on the drawings are a combination of a historic Cardno topographic survey, and BeforeUDig plans received from utility operators.

Investigation works for key strategic services is to be undertaken during further design phases.

Due to the history of the site there is the possibility underground services are present that are live and not recorded by network utility operators (NUOs). After surface services mark outs, site stand over by NUOs will be required throughout the construction works to confirm whether exposed services are redundant or need to be protected.

Aurecon makes best endeavours to locate existing services as part of its design scope; however it cannot be held liable for the accuracy of existing services plotted by others.

# 1.5 Scope of Civil Works

The civil engineering scope associated with this report is primarily in relation to civil services, drainage and flooding. The details of the precinct finishes are included in the landscape architect design (by others).

Aurecon civil engineering concept drawings are provided in Appendix A.

### 1.6 Wellington Water correspondence

Please refer to Appendix B for records of Aurecon correspondence with Wellington Water. A summary of key outputs of engagement with Wellington Water is as below:

- Wellington Water indicate that there is >10 L/s spare capacity in the local 450mm diameter wastewater pipe, and ~29 L/s capacity in the local 250mm diameter wastewater network on Wakefield Street.
- The area (including the site) is prone to flooding, with >500mm depth of flood water predicted during a 1 in 100-year ARI storm event. The site sits within an overland flow path, and its construction may cause up to 30mm additional flood depth due to the displacement of flood water (refer to report Section Flood Risk)
- The existing water supply in Wakefield Street is likely to have 75-80m of pressure head, however further confirmation of demands is required in consultation with the fire engineer. This will be confirmed through planned hydrant testing.
- There is an existing large ovoid stormwater culvert that runs underneath the east of the site, and under the proposed building footprint that is required to be protected or diverted. (This work is being undertaken in conjunction with the structural engineer for the project). Further correspondence with Wellington Water dated 22<sup>nd</sup> February 2022 indicates that Wellington Water's approval to construct over this culvert is uncertain.
- There is an existing large wastewater buffer storage tank within the site that is located partially under the proposed building footprint that is required to be protected. (This work is being undertaken in conjunction with the structural engineer for the project). Further correspondence with Wellington Water dated 22<sup>nd</sup> February 2022 indicates that Wellington Water's approval to construct over the wastewater tank is uncertain.

# 2 Existing Services

### 2.1 Network Utility Operator Services

Aurecon have received Before U Dig records of utilities from network utility operators (NUOs) near and within the site. A summary of the existing NUO services are provided in Appendix C.

# 2.2 Private Site Services

A number of private site services are present on the site, generally to service the existing RNZB building and the adjacent Michael Fowler Centre. These services are shown on Aurecon's Existing Services drawing and are sourced from RNZB design drawings and the historic Cardno survey.

# 2.3 Future Investigations

The following future existing services investigations are recommended:

- CCTV survey RNZB 100mm dia sewer lateral to public drainage in Wakefield St, to enable potential replacement on same alignment
- Accurately locate existing ovoid stormwater culvert to the to enable piling design
- Accurately locate existing wastewater tank services to avoid clashes
- Accurately locate existing gas mains in Wakefield St footpath to avoid clashes with foundation excavations
- Accurately locate existing power cables in Wakefield St footpath to avoid clashes with foundation excavations
- Surveying invert levels in relevant manholes to enable drainage connection design

These investigation recommendations are captured on Aurecon's existing services drawing (refer Appendix A).

# 3 Stormwater Drainage

### 3.1 Existing Drainage Services

Wellington City Council records for stormwater assets are shown in the Figure below.



Figure 2: Existing public stormwater infrastructure (Source: Wellington Water's Regional Water Stormwater Wastewater App)

A large ovoid stormwater main runs beneath the eastern side of the site and across Jervois Quay to Wellington Harbour. Smaller mains, on the north east side of the RNZB building and the south side of the RNZB building, sized 375mm and 300mm diameter respectively, are located within the site. A private stormwater pipe of unknown size is located on the eastern side of the Michael Fowler Centre (MFC) building discharging into the 375mm main. Existing catchpits within the site provide inlets to the stormwater system and connect to the existing 300mm main.

The large ovoid stormwater main has been inspected using CCTV, in accordance with the New Zealand Gravity Pipe Inspection Manual 4<sup>th</sup> Ed. The main appeared to be in moderate condition, with isolated areas of wear or corrosion. The main appeared to be of concrete construction, whereas it is noted as brick construction in the Wellington Water GIS system. A small number of laterals directly discharging into the pipe will need to be properly sealed during the works. The factual inspection report associated with the CCTV survey is provided in Appendix E.

Email correspondence from Wellington Water dated 22 February 2022 indicates acceptance to construct the building over the chamber from Wellington Water is not certain. Engagement with Wellington Water is

ongoing, noting that the range of solutions potentially have a significant impact on civil in-ground works. Design mitigations for constructing the building over this stormwater main are addressed in the Dunning Thornton report *Structural Effects and Construction Methodology (dated 27<sup>th</sup> January 2022)*.

The 375mm concrete lateral entering the large stormwater main has been CCTV surveyed; the pipe is in moderate condition and enters the large stormwater main part-way up the height of the chamber WCC\_SW024290. Detailing for potential re-use of this connection will be developed in further design stages. The factual inspection report associated with the CCTV survey is provided in Appendix E.

The site is already predominantly surfaced with impervious materials, with the exception of the eastern park.

A low concrete nib wall on the boundary with Jervois Quay is likely to contain stormwater within the site, in the existing scenario.

# 3.2 Proposed Drainage Works

The existing public 300mm and 375mm dia mains in the northern portion of the site are planned to be upgraded on a new alignment to accommodate excavations and foundations for the building, as shown on the supplied proposed services plan. We note the proposed design alignment is focused on minimising possible clashes with existing tree roots. A portion of the existing network may be able to be maintained adjacent to the new building, as outlined in the Proposed Services drawing. This would require temporary excavation retention and further accurate location of the mains and coordination with pile locations. These mains will be constructed in accordance with Wellington Water requirements. The existing connection to the ovoid stormwater pipe is intended to be used, de-risking the connection to this older infrastructure. These mains will be constructed in accordance with Wellington Water requirements.

Changes to the impervious area has been checked by the landscape architects and is confirmed to be no greater than existing. On this basis the development is expected to be hydraulically neutral. The assessment of impervious areas is provided in Appendix D.

Private site drainage networks are required for the redeveloped public walkway which connects Cuba Street with Jervois Quay, as well as drainage in landscape areas. Provisional catchpits and subsoil drains have been allowed for this; however the levels and grades for the public walkway area are still to be developed which will determine the final drainage layout.

# 4 Flooding

### 4.1 Introduction

An initial flood risk assessment has been undertaken for the proposed building.

The flood risk assessment includes:

- Computing peak flood levels
- Identifying impacts of the proposed development on flood levels in the wider catchment
- Recommending minimum building floor level

This section summarises the methodology and results of the assessment.

# 4.2 Methodology

A copy of the hydraulic model of the Southern CBD Stormwater Catchment has been obtained from Wellington Water and has been used for the assessment.

- Model: Southern CBD Existing ICM v10.5
- Network: SouthernCBD\_Existing

- Network Version: 2
- Network Scenario: Working Base Version

Simulations have been run for:

- 100yr ARI (Average Recurrence Interval) current climate,
- 100yr ARI including allowances for the impact of climate change (both rainfall intensity and sea level rise).

The simulations are based on the 12hr design storm event.

No changes have been made to the model representing the existing development of the Southern CBD in Wellington.

The impact of the proposed building has been assessed by blocking out the ground level building footprint as shown in Figure 3 below.

Figure 3 Extent of ground level footprint



All levels are presented in Wellington Vertical Datum 1953.

### 4.3 Assumptions

The modelling is based on the following assumptions:

- Rainfall: A 12hr rainfall profile was adopted representing a 10 year and 100yr ARI design storm event.
- Tide data: A dynamic tidal boundary was applied with a peak of 1.1mRL.
- Climate Change (CC):

The model includes a climate change scenario which assuming 20% increase in rainfall and 1m sea level rise.

Ground profile:

The ground profile is represented by a 1m x 1m Digital Elevation Model (DEM) derived from LiDAR data collected in 2013. Large buildings have been blocked out.

RBNZ building:

The existing temporary Royal Ballet of New Zealand (RBNZ) building is not represented in the Southern CBD model (for both the Existing and the MFC Proposed development scenarios).

More detailed modelling assumptions are reported in Southern CBD Stormwater Catchment – Model Build Report<sup>1</sup>.

### 4.4 Model Run Results

The modelled design storm simulations are presented in Table 1 below.

 Table 1
 Modelled Simulations and Results

Design Event	Rainfall Depth	Peak Sea Level
10yr ARI	78mm	1.1m RL
10yr ARI + CC	93mm	2.1m RL
100yr ARI	122mm	1.1m RL
100yr ARI + CC	147mm	2.1m RL
Sensitivity Run: 100yr ARI + CC (rainfall only)	147mm	1.1m RL

The flood depth and extent for the 100yr ARI + CC design event is presented in Figure 4 below. The flood depth is up to 0.6m along the Wakefield Street side of the building.

<sup>&</sup>lt;sup>1</sup> Southern CBD Stormwater Catchment – Model Build Report, Wellington Water, Rev 1, Jul 2019



Table 2 below shows the modelled peak flood levels near the building for both the Existing Development and MFC Proposed Development. The results show that the impact of the building on the flood levels in the vicinity is an increase of 0.02m. It is noted that these levels do not include an allowance for freeboard.

Table 2 Modelled Simulations and Results

Design Event	Rainfall	Peak Tide Level (in m WVD-53)	Computed Pea (in m W	Variance	
			Existing Development	Proposed Development	(in m)
10yr Existing Climate	10yr ARI	1.1m WVD-53	2.39	2.40	+0.01
10yr Incl Climate Change	10yr ARI + 20%	2.1m WVD-53	2.58	2.60	+0.02
100yr Existing Climate	100yr ARI	1.1m WVD-53	2.61	2.63	+0.02
100yr Incl Climate Change	100yr ARI + 20%	2.1m WVD-53	2.80	2.83	+0.03
Sensitivity 100yr + 20% Rain	100yr ARI + 20%	1.1m WVD-53	2.66	N.A.	N.A.

Note: The above levels are the computed peak flood levels at Wakefield Street near the entrance to the carpark excluding freeboard.

### 4.5 Flood impact

The increase in modelled peak flood levels between the Proposed Development and Existing Development scenarios is shown in Figure 5 and Figure 6 for the 10yr+CC and 100yr+CC design events respectively.



The maps show an increase up to 20mm for the 10yr ARI + CC event and up to 30mm for the 100yr ARI + CC event.

It is noted that there is quite a bit of "noise" (i.e. areas showing a dense mixture of positive and negative values) due to the difference in mesh element sizes and shapes between the modelled scenarios and the small magnitude of variance presented (i.e. margin of error). This is a common modelling issue and these areas should be ignored as they do not show a consistent pattern.

- In general terms the following can be concluded: Flood impact is mainly limited to Wakefield Street (from Taranaki Street to Victoria Street)
- The variances at Manners Street and Courtenay Place are considered to be outside the flood impact area and should therefore be ignored.
- The impact is up to 20mm during the 10yr+CC and 30mm during the 100yr+CC event.





The existing floor levels of buildings within the affected area has been surveyed. The surveyed floor levels are shown in Figure 7 below. The figure shows that all buildings are already expected to flood during a 100-year ARI + CC design event up to around 0.7m. The increase in flood levels of 30mm is considered a minor impact on existing properties.



### Figure 7 Surveyed Floor Levels and Proposed Development Flood Levels 100yr ARI + CC

# 4.6 Freeboard and Minimum Finished Floor Level

Following discussion with Wellington Water a freeboard of 0.2m is required to the top of foundation slab. This freeboard is to allow for:

- physical processes that may not have been allowed for in the model
- uncertainties in the precision of the hydraulic modelling
- and uncertainties in the prediction of physical processes.

Discussions with Wellington Water and Wellington City Council has confirmed that for a base isolated building the freeboard may be taken to the top of the foundation slab, being not subject to water damage.

The minimum Finished Floor Level (FFL) is therefore to be 3.03 m RL per WVD-53 or higher, being 200mm above the 100-year ARI + Climate Change flood level of 2.83 m RL.

Water Supply 5

# 5.1 Existing infrastructure

Wellington City Council records show a 200mm diameter Ductile Iron water main within Wakefield Street on the far side of the road to the development. A 250mm diameter Ductile Iron water main is also present in Cuba St which crosses Wakefield St and enters the western side of the Michael Fowler Centre site between the MFC and the town hall building, eventually connecting to mains in Jervois Quay and Civil Square. A 150mm water main is present in the southern side of Jervois Quay. Larger 450mm and 800mm transmission mains are present in Jervois Quay; however these would be unavailable for private connections. Several 20mm dia supplies to the site are shown on the GIS; furthermore a 40mm supply to the RNZB building is

known to be provided from Wakefield St (not shown on Figure). The existing water infrastructure records from Wellington Water are shown in the below Figure.

![](_page_16_Picture_1.jpeg)

Figure 8: Council records of water infrastructure (Source: Wellington Water's Regional Water Stormwater Wastewater App)

Wellington Water modelling has shown a pressure head of 75-80m in the existing mains on Wakefield Street.

### 5.2 **Proposed water supply infrastructure**

Water supplies will be required for potable and sprinkler use in the new building.

Peak demand for the sprinkler system is estimated to be 52.6 L/s as per NZS 4541. A 150mm diameter Ductile Iron Cement Lined pipe from the existing water main in Wakefield Street is proposed to service this demand as shown on the supplied proposed services plan

Potable water demand is estimated to be in the order of 3.1 L/s as per the CIPHE Plumbing Engineering Services Design Guide. A DN90 HDPE is proposed to service the potable demand.

Wellington Water have made an initial assessment of the public supply in Wakefield St and have advised that there is a modelled 75-80m head in the line. This is likely to provide ample supply for the peak demands with the required residual head; however, hydrant testing is planned to confirm this.

Backflow prevention is to be provided by the building services trades, within the building footprint.

The 20mm site supplies are planned to be retained for irrigation of the planting areas.

Separation of potable and sprinkler supplies, to meet any security of supply requirements, are to be confirmed in further stages of design.

# 6 Wastewater Drainage

# 6.1 Existing System

Wellington City Council records show public wastewater drainage networks in the surrounding area as per the below Figures.

![](_page_17_Picture_3.jpeg)

Figure 9: Council records of wastewater infrastructure (wide area view) (Source: Wellington Water's Regional Water Stormwater Wastewater App)

![](_page_17_Figure_5.jpeg)

Figure 10: Council records of wastewater infrastructure in Wakefield St (Source: Wellington Water's Regional Water Stormwater Wastewater App)

A 450mm diameter concrete drainage pipe services Wakefield St and its upstream catchment, draining into the Pump Station shown at the intersection of Taranaki St, Market Lane, and Jervois Quay. A second, smaller public network (typically 250mm dia) collects wastewater from some buildings in the Wakefield Street and Cuba Streets area, discharging into a pump station in the south-west corner of the Michael Fowler Centre site. A pressurised rising main from the pump station is also present in Wakefield Street.

# 6.2 Wastewater Storage Chamber

A buried wastewater storage chamber is present below ground within the site, and it is proposed to construct the building footprint partially over the footprint of the chamber.

The chamber condition was inspected on 2 December 2021, comprising:

- Visual condition assessment of structure
- Hammer test to detect any areas of poor concrete quality.

The chamber appears to be in good condition, with no areas of deterioration detected.

Design, construction, and operational mitigations for constructing over this chamber are addressed in the Dunning Thornton report *Structural Effects and Construction Methodology (dated 27th January 2022).* 

No changes to the hydraulic operation of the chamber are proposed.

Email correspondence from Wellington Water dated 22 February 2022 indicates acceptance to construct the building over the chamber from Wellington Water is not certain. Engagement with Wellington Water is ongoing, noting that the range of solutions potentially have a significant impact on the design of the building.

# 6.3 Proposed Drainage Works

A single 150mm diameter lateral connection to the existing public 250mm network is proposed as shown on the supplied proposed services plan. Wellington Water has confirmed this network has capacity for the expected peak wet weather discharge of 9 L/s. This would likely be on the same alignment as the existing RNZB sewer lateral (i.e. replacing the lateral with a larger pipe).

# 7 Other Services

High voltage power is expected to be drawn from existing HV lines beneath Wakefield St, to a substation within the ground floor of the proposed building or located externally on the berm beside the building. Design development discussions with Wellington Electricity are ongoing.

Fibre-optic connections are to be from the Chorus network located in Wakefield Street. Design development regarding connection locations is ongoing.

Existing gas mains of up to 100mm diameter are present in the footpath of Wakefield Street, immediately adjacent to the proposed building. Design of the building foundations and construction methodologies will need to account for the presence of this gas main. Positive identification and accurate survey of the gas main will be required as an input to the design and is to be undertaken during further design stages.

Public streetlights are to be relocated to the road edges of the footpaths in Wakefield Street and Jervois Quay, in consultation with Wellington City Council.

Lighting within the site shall be designed in accordance with NZS1158.3.1:2020. Preliminary lighting levels have been selected in accordance with this standard and are shown on the Landscape Architect's drawings.

# 8 Sustainability

The external works design comprises features which are intended to contribute to a highly sustainable building, to be demonstrated through a 5-star NZ Green Star rating. The specific NZ Green Star credits being targeted in the civil design are:

• 25.1 Stormwater Peak Discharge:

1 point is available where the post-development peak Average Recurrence Interval (ARI) event discharge from the site does not exceed the pre-development peak ARI event discharge.

25.2 Stormwater Pollution Targets:

1 additional point is available, where the first point has been achieved and all stormwater discharged from site meets specified pollution reduction targets.

'Stormwater Peak Discharge' is likely to be met as the impervious area on the site is proposed to be reduced compared with the pre-development impervious area by introducing a net increase in soft landscaped areas, including areas at the north west of the site as part of a wider precinct improvement plan.

'Stormwater Pollution Targets' is likely to be met as:

- The paved area which will be frequently trafficked by vehicles will be reduced, thereby reducing contaminant load from vehicles tyres, brakes and the like
- The number of vehicle movements on the site will be reduced, by deletion of car parking. Traffic will typically be service vehicles to the adjacent Michael Fowler Centre, and minor light vehicle access.
- Green landscape areas will help to capture runoff from trafficked paved areas.
- A stormwater filtration device(s) may be provided, should trafficked paved areas not be able to be treated by other means. This is also subject to ownership agreements of drainage assets with Wellington City Council.

The design is at an early stage with some elements to be confirmed; however we are confident that the requirements for thee credits can be met as the project advances.

# 9 Safe Design

Aurecon is committed to safe delivery of the proposed building. The key goal is to provide safer design for construction, operation, maintenance, and demolition phases throughout the project's lifecycle. Aurecon adopts a risk-based approach to eliminate or mitigate hazards not fully treated by the reference regulations, standards, codes, and guidelines.

The safe design process used on this project is based on a risk management approach. The safe design process:

- Maximises the likelihood of project safety objectives being achieved; and
- Documents the safe design work undertaken; and
- Records associated outcomes.

This section provides information on health and safety aspects of the design undertaken by Aurecon in the delivery of our services. At an appropriate point we recommend these risks are considered as part of a wider Safe Design workshop with the Client, Designers, and stakeholders.

Safe Design issues will continue to be assessed and mitigated throughout the design development. These include safety implications that impact that installation, construction, operations and maintenance of the proposed design. Key safety in design items that relate to the concept civil engineering works design thus far are:

• **High risk underground services**. The works will require excavation adjacent to strategic consumer gas assets for forming of foundations, crawl spaces, and the like. These works will require accurate location of the services and choosing structural design solutions which minimise risk to the existing services.

Further, the risk will always remain as there may be other unknown services within the site, so great care must be taken during construction.

- **Pedestrian traffic.** Significant pedestrian traffic moves through the site currently between Cuba Street and the Waterfront. This will need to be managed with appropriate staging of works.
- **Excavations**. Common coordinated service trenches are proposed to minimise excavations to dedicated routes through the site and minimise disruption in the public realm, where possible.
- Flood Risk. Site specific flood modelling has been undertaken using WWLs existing catchment model to
  assess the effects of the building on the overland flow path, flow velocities, depths, the proposed finished
  floor level, and impacts on surrounding buildings. The maximum increase in flood levels is relatively
  modest and will be assessed against existing building threshold levels to determine appropriate
  mitigation, if any is required.
- Existing 3 waters infrastructure. Significant work to determine the condition of existing buried wastewater and stormwater assets on the site is being undertaken to ensure that the design of the building is adapted to protect these existing structures to ensure no detrimental operational or structural effects from the development. (Please refer to the structural engineers report for further information on the protection measures proposed).

# Appendix A Drawings

![](_page_22_Figure_0.jpeg)

![](_page_23_Figure_0.jpeg)

Appendix B Wellington Water Correspondence

### **Alice Hoskins**

Subject:	Wastewater Capacity on Wakefield St
Location:	Microsoft Teams Meeting
Start:	Wed 23/02/2022 12:30 PM
End:	Wed 23/02/2022 1:00 PM
Recurrence:	(none)
Meeting Status:	Meeting organizer
Organizer:	Alice Hoskins
Required Attendees:	Mohammed.Hassan@wellingtonwater.co.nz

# Microsoft Teams meeting

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945682022@t.plcm.vc Video Conference ID: 131 699 694 9 <u>Alternate VTC instructions</u>

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Phone Conference ID: 840 016 216# Find a local number | Reset PIN

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### **Brock Goodison**

Subject:

RE: Building over drains/storage tank - Car Park Flood Sources and Levels-wwl -18 Sep 2021(address:111 Wakefield Street, Te Aro)

From: Samir Hermiz <<u>Samir.Hermiz@wellingtonwater.co.nz</u>>

Sent: Tuesday, 22 February 2022 9:26 am

To: Graham Nash <<u>Graham.Nash@aurecongroup.com</u>>

Cc: Olena Chan <<u>Olena.Chan@wellingtonwater.co.nz</u>>; Mohammed Hassan

<<u>Mohammed.Hassan@wellingtonwater.co.nz</u>>; Steve Hutchison <<u>Steve.Hutchison@wellingtonwater.co.nz</u>>; Tim Strang <<u>Tim.Strang@wellingtonwater.co.nz</u>>; Sam Lister <<u>Sam.Lister@wellingtonwater.co.nz</u>>; Sean de Roo <<u>Sean.deRoo@wellingtonwater.co.nz</u>>; Sheena O'Brien <Sheena.O'Brien@wellingtonwater.co.nz>; Zac Jordan <<u>Zac.Jordan@wcc.govt.nz</u>>; Charles Puha <<u>Charles.Puha@wellingtonwater.co.nz</u>>; Samir Hermiz <<u>Samir.Hermiz@wcc.govt.nz</u>>;

Subject: RE: Building over drains/storage tank - Car Park Flood Sources and Levels-wwl -18 Sep 2021(address:111 Wakefield Street, Te Aro)

Importance: High

### Hi Graham

Thank you for your email after the discussion and meeting with different department in Wellington Water Ltd, please see the below comments and requirements;

- 1. The proposed building must comply with the Regional Standard for Water Services December 2021 (rev.3) and the Regional Specification for Water Services December 2021 (rev.3).
- 2. Building over the existing underground wastewater tank is not accepted in this case because the following reason:
- A. WWL needs 24/7 access for heavy vehicles and this must be kept open.
- B. As a minimum, an easement should be granted in favour of Wellington Water Ltd to ensure that the access hatches and reasonable surrounding area is accessible 24/7 and that no significant landscaping or similar work is done on or around those critical access points that hinders that access.
- C. The building vehicle and pedestrian entrance arrangements need to be carefully reviewed and should be moved away from this utility access area.
- D. The building tenants will need to recognise that this is a key operating infrastructure facility which is prone to noisy and smelly maintenance from time to time and to manage their own activities accordingly.
- E. With longer term considerations, our ability to repair / refurbish / renew this asset would be severely compromised by this proposal. For example, seismic strengthening or earthquake repairs would normally be undertaken by opening the top of the structure. Having to access from one end creates a more hazardous confined space for workers, limits the use of heavy equipment and will inevitably increase the cost of such work. For example, in the unfortunate event that the structure was damaged in a severe earthquake it would be extremely difficult to repair with a large building over half the footprint.

Therefore the proposed building should be relocated away from this asset.

3. Building over the existing public stormwater culvert :

- A. According to GIS records, the culvert is over 100 years old and there is evidence of spalling and erosion damage. It's highly likely the culvert will need replacement or rehabilitation within the life of the building.
- B. The dimension of the culvert would make person-entry and insitu repairs difficult and limited in scope.
- C. Replacement of the culvert with a circular pipe is unlikely to perform hydraulically and may create unacceptable maintenance issues over the longer term.
- D. Diversion of the culvert around the building in the future does not look to be feasible given the dimensions of the culvert and downstream constraints.
- E. If a section of the culvert fails, then options for rehabilitation may be limited without severely compromising hydraulic performance.
- F. This culvert provides the primary stormwater conveyance from Aro Valley and the adjacent CBD areas. During heavy rainfall it will be flowing completely full and pressurised to above ground level. Any reduction in capacity will directly increase upstream and adjacent flooding issues.
- G. Replacement with a rectangular culvert may be feasible but connections between the culverts and appropriate transitioning would need to be worked through.
- H. Given these circumstances, the culvert will need to be replaced prior to the construction of the building going ahead. Any alterations or additions to existing Public Drains will require Construction Plans to be submitted to the Land Development Team, Wellington Water, for approval. The work must be carried out by a Registered Drainlayer who is employed by a contractor who has an approved Health and Safety Plan and Public Liability Insurance. All work is to be carried out in accordance with the approved plans, the Wellington City Council Code of Practice for Land Development, Regional Standard for Water Services December 2021, and a Public Drainage Permit (As distinct

Standard for Water Services December. 2021, and a Public Drainage Permit (As distinct from a Building Consent ) to be issued by the Public Drainage Team and fees paid.

Please revise the proposed plans to comply with the above requirements.

Please feel free to contact me if you need more information.

Regards, ,

Samir Hermiz Drainage Engineer - Land Development

![](_page_27_Picture_13.jpeg)

Tel +64 4 912 4466 DDI Mob 021 879 133

Private Bag 39804, Wellington Mail Centre 5045 Level 4, IBM House, 25 Victoria Street, Petone, Lower Hutt www.wellingtonwater.co.nz

From: Graham Nash <<u>Graham.Nash@aurecongroup.com</u>>

Sent: Wednesday, 16 February 2022 11:40 am

To: Samir Hermiz <<u>Samir.Hermiz@wellingtonwater.co.nz</u>>; Mohammed Hassan

<<u>Mohammed.Hassan@wellingtonwater.co.nz</u>>; Paul Winstanley <<u>Paul.Winstanley@wellingtonwater.co.nz</u>>; Sean de Roo <<u>Sean.deRoo@wellingtonwater.co.nz</u>>; Sheena O'Brien <Sheena.O'Brien@wellingtonwater.co.nz> **Cc:** Brock Goodison <<u>Brock.Goodison@aurecongroup.com</u>>; Kerrin Manuel <<u>KerrinM@mcguinness.co.nz</u>>; Adam Thornton <<u>adam.thornton@dunningthornton.co.nz</u>>; Jamie Reeve <<u>Jamie@willisbond.co.nz</u>>; Rosalind Luxford (<u>Rosalind@willisbond.co.nz</u>) <<u>rosalind@willisbond.co.nz</u>>

Subject: RE: Building over drains/storage tank - Car Park Flood Sources and Levels-wwl -18 Sep 2021(address:111

Wakefield Street, Te Aro) Importance: High

Hi Samir,

Further to our teams meeting and the below, and subsequent sharing of the basement draft model, can you confirm what comments WWL has please, or if you are satisfied with the information provided at this stage?

Thank you

**Graham Nash** BSc(Dist) IEng MICE Associate, Land Infrastructure, Aurecon **T** +64 4 4718768 **M** +64 27 2516079

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DISCLAIMER

From: Graham Nash

Sent: Tuesday, 1 February 2022 4:27 pm

To: 'Samir Hermiz' <<u>Samir.Hermiz@wellingtonwater.co.nz</u>>; 'Mohammed Hassan'

<<u>Mohammed.Hassan@wellingtonwater.co.nz</u>>; 'Paul Winstanley' <<u>Paul.Winstanley@wellingtonwater.co.nz</u>>; 'Sean de Roo' <<u>Sean.deRoo@wellingtonwater.co.nz</u>>; 'Sheena O'Brien' <Sheena.O'Brien@wellingtonwater.co.nz> **Cc:** Brock Goodison <<u>Brock.Goodison@aurecongroup.com</u>>; 'Kerrin Manuel' <<u>KerrinM@mcguinness.co.nz</u>>; Adam Thornton <<u>adam.thornton@dunningthornton.co.nz</u>>; 'Jamie Reeve' <<u>Jamie@willisbond.co.nz</u>>; 'Rosalind Luxford (<u>Rosalind@willisbond.co.nz</u>)' <<u>rosalind@willisbond.co.nz</u>>;

**Subject:** RE: Building over drains/storage tank - Car Park Flood Sources and Levels-wwl -18 Sep 2021(address:111 Wakefield Street, Te Aro)

Hi Samir and wider WWL team,

Thanks for your time this morning – much appreciated. Please see attached my notes and actions from today's meeting.

Note: Adam is going to forward on some additional drawing perspectives as requested over the next few days

If I have omitted anything critical please advise – we look forward to WWLs comments, and are happy to catch up prior to the 15<sup>th</sup> Feb if you wish to discuss any design aspects.

#### Regards

**Graham Nash** BSc(Dist) IEng MICE Associate, Land Infrastructure, Aurecon **T** +64 4 4718768 **M** +64 27 2516079

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#### DISCLAIMER

#### From: Graham Nash

Sent: Tuesday, 1 February 2022 10:48 am

**To:** Samir Hermiz <<u>Samir.Hermiz@wellingtonwater.co.nz</u>>; Rosalind Luxford (<u>Rosalind@willisbond.co.nz</u>) < <u>rosalind@willisbond.co.nz</u>>; Jamie Reeve <<u>Jamie@willisbond.co.nz</u>>; Adam Thornton

<<u>adam.thornton@dunningthornton.co.nz</u>>; Mohammed Hassan <<u>Mohammed.Hassan@wellingtonwater.co.nz</u>>; Paul Winstanley <<u>Paul.Winstanley@wellingtonwater.co.nz</u>>; Sean de Roo <<u>Sean.deRoo@wellingtonwater.co.nz</u>>; Sheena O'Brien <Sheena.O'Brien@wellingtonwater.co.nz>

**Subject:** RE: Building over drains/storage tank - Car Park Flood Sources and Levels-wwl -18 Sep 2021(address:111 Wakefield Street, Te Aro)

FYI folks

#### Cheers

**Graham Nash** BSc(Dist) IEng MICE Associate, Land Infrastructure, Aurecon **T** +64 4 4718768 **M** +64 27 2516079

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DISCLAIMER -----Original Appointment-----From: Samir Hermiz <<u>Samir.Hermiz@wellingtonwater.co.nz</u>> Sent: Thursday, 27 January 2022 11:51 am To: Samir Hermiz; Rosalind Luxford (<u>Rosalind@willisbond.co.nz</u>); Jamie Reeve; Graham Nash; Adam Thornton; Mohammed Hassan; Paul Winstanley; Sean de Roo; Sheena O'Brien Subject: FW: Building over drains/storage tank - Car Park Flood Sources and Levels-wwl -18 Sep 2021(address:111 Wakefield Street, Te Aro) When: Tuesday, 1 February 2022 10:00 am-11:00 am (UTC+12:00) Auckland, Wellington. Where: Microsoft Teams Meeting

----Original Appointment----From: Samir Hermiz
Sent: Thursday, 27 January 2022 11:47 am
To: Adam Thornton; Mohammed Hassan; Paul Winstanley; Sean de Roo; Sheena O'Brien
Subject: Building over drains/storage tank - Car Park Flood Sources and Levels-wwl -18 Sep 2021(address:111
Wakefield Street, Te Aro)
When: Tuesday, 1 February 2022 10:00 am-11:00 am (UTC+12:00) Auckland, Wellington.
Where: Microsoft Teams Meeting

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### **Alice Hoskins**

From:	Land Development <land.development@wellingtonwater.co.nz></land.development@wellingtonwater.co.nz>
Sent:	Thursday, 17 February 2022 9:58 AM
То:	Alice Hoskins
Cc:	Brock Goodison; Land Development
Subject:	FW: Michael Fowler Centre - Pipe capacities and unknown utilities locations (SW)

#### Hi Alice

The estimated/modelled for the Level of Service (return period) of stormwater pipes in the area is colour coded.

![](_page_31_Figure_4.jpeg)

![](_page_31_Picture_5.jpeg)

I will send you wastewater information once I receive from our modeller.

regards
Mohammed Hassan Team Leader Land Development Consenting

![](_page_31_Picture_8.jpeg)

Tel 04 912 4400 DDI 049124591 Mob 021913809 Private Bag 39804, Wellington Mail Centre 5045 Level 4, IBM House, 25 Victoria Street, Petone, Lower Hutt www.wellingtonwater.co.nz

Wellington Water is owned by the Hutt, Porirua, Upper Hutt and Wellington city councils and Greater Wellington Regional Council. We manage their drinking water, wastewater and stormwater services. From: Alice Hoskins <<u>Alice.Hoskins@aurecongroup.com</u>>
Sent: Tuesday, 15 February 2022 4:26 pm
To: Land Development <<u>Land.Development@wellingtonwater.co.nz</u>>
Cc: Brock Goodison <<u>Brock.Goodison@aurecongroup.com</u>>
Subject: Michael Fowler Centre - Pipe capacities and unknown utilities locations

#### Hi WW,

We are designing a large scale building on the existing ground of the Michael Fowler Carpark. We are needing to know the capacities of the surrounds WW and SW. Below is an image from your GIS system of the building we are looking out (115 Wakefield Street).

We have two main queries;

- 1. The capacities of the WW and SW networks in the area
- 2. Are there any additional info that does not appear in this GIS view?

We ask the second due to a previous investigation in which we believe there might be a WW storage tank near the right PS near the entrance to the carpark. If this is the case, it might affect construction traffic etc. Also if you look at the zoomed in figure, there are several SW pipes that appear to not be connected to anything. Would you know if this is private infrastructure we need to follow up on?

![](_page_33_Picture_0.jpeg)

Finally, we went out to site an found infrastructure on site that did not appear in the GIS. Such as several (what looked like SW or WW manholes) up at the top right (in black circles) and a marked out WW line including clear previous construction trench lines (in the red colour down the bottom of the page). We want to confirm that this is in your data base? Are they new connections? Are the SW ones up the top decommissioned?

![](_page_34_Figure_0.jpeg)

We are just wanting to gain better understanding of the underground services in the area so we do not incur any surprises or instances.

Kind regards,

Alice Hoskins Civil Engineer, Aurecon

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Level 2, lwikau Building 93 Cambridge Terrace Christchurch New Zealand 8013 <u>aurecongroup.com</u>

![](_page_35_Picture_0.jpeg)

### MEMO

TOLand developmentCOPIED TOFROMAhmed HassanDATE13 September 2021SUBJECTWastewater development impact assessment of MFC car park, Te AroFOR YOUR INFORWATION

### Asset impact assessment

DETAILS			
Address	MFC car park, Te Aro, WELLINGTON		
Source	Email dated 09 September 2021		
Туре	Development enquiry		
Reference			
Asset	Wastewater		
MODEL			
--------------	---	---------	------
Software	InfoWorks ICM	Version	10.5
Model Name	WCC Moa Point WwTP	Version	5
Base Year	2015 (current)		
Design Event	1-year LTS (Moa Point 1Y > Kelburn with evap)		

#### LOCAL NETWORK

Current Capacity

/

N/A



manhole WW38599 that discharges via the Taranaki St stormwater system into Wellington Harbour. It is unclear from the model results how frequently it operates

#### SUMMARY

The trunk network to PS005 has at least 10 litres/sec of spare design capacity during the same event. There is an engineered overflow that discharges via the Taranaki St stormwater system into Wellington Harbour. It is unclear from the model results how frequently it operates

#### DISCLAIMER

This assessment is based on the results from WWL hydraulic models as defined in this memorandum. It does not take into account the impact on the spare design capacity of other developments that have occurred since then, are currently underway, or possible future developments. Non-hydraulic parameters like pipe age, conditions and likelihood of their failure have not been assessed. Flow monitoring may be required to verify these results. This development may impact on the spare design capacity available for possible future developments along the downstream network.

### **Brock Goodison**

From:	Modelling Team <modelling.team@wellingtonwater.co.nz></modelling.team@wellingtonwater.co.nz>
Sent:	Monday, 13 September 2021 10:26 AM
То:	Graham Nash; Land Development
Subject:	RE: MFC - Car Park Flood Sources and Levels-wwl - 7 Sep 2021

Hi Graham,

Please find the below WW assessment for this site

### Asset impact assessment

DETAILS	
Address	MFC car park, Te Aro, WELLINGTON
Source	Email dated 09 September 2021
Туре	Development enquiry
Reference	
Asset	Wastewater

MODEL			
Software	InfoWorks ICM	Version	10.5
Model Name	WCC Moa Point WwTP	Version	5
Base Year	2015 (current)		
Design Event	1-year LTS (Moa Point 1Y > Kelburn with evap)		

LOCAL NETWORK					
Current Capacity	N/A				



#### SUMMARY

The trunk network to PS005 has at least 10 litres/sec of spare design capacity during the same event. There is an engineered overflow that discharges via the Taranaki St stormwater system into Wellington Harbour. It is unclear from the model results how frequently it operates

#### DISCLAIMER

This assessment is based on the results from WWL hydraulic models as defined in this memorandum. It does not take into account the impact on the spare design capacity of other developments that have occurred since then, are currently underway, or possible future developments. Non-hydraulic parameters like pipe age, conditions and likelihood of their failure have not been assessed. Flow monitoring may be required to verify these results. This development may impact on the spare design capacity available for possible future developments along the downstream network.

Ngā mihi Ahmed

Ahmed Hassan Senior Hydraulic Modelling – Land Development



Tel 04 912 4400 Mob 021 354 764 Private Bag 39804, Wellington Mail Centre 5045 Level 4, IBM House, 25 Victoria Street, Petone, Lower Hutt www.wellingtonwater.co.nz

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From: Graham Nash <Graham.Nash@aurecongroup.com>
Sent: Thursday, 9 September 2021 5:45 pm
To: Sarah Zhou <Sarah.Zhou@wellingtonwater.co.nz>
Cc: Modelling Team <Modelling.Team@wellingtonwater.co.nz>; Ahmed Hassan
<Ahmed.Hassan@wellingtonwater.co.nz>
Subject: RE: MFC - Car Park Flood Sources and Levels-wwl - 7 Sep 2021

Hi Sarah,

Further the below can I add two other points/queries please in the interim if you can advise:

 Flood levels with respect to FFLs. The regional standard below indicates freeboard is measure to from the underside of joists or concrete slab. Can I confirm that the underside reference was only for timber joist supported floors, and does not apply to a concrete slab on grade. I.e. that freeboard is measured to the top of the concrete slab. Thanks

4.2.9 Freeboard

Unless Wellington Water has undertaken a formal assessment of an appropriate freeboard allowance based on sensitivity testing in a validated hydraulic model, habitable building floors shall have a freeboard of 500 mm above the surface water of the secondary level of protection event. Commercial and industrial buildings shall have a freeboard of 300mm and all other building freeboards shall be 200 mm.

The minimum freeboard shall be measured from the top of the peak water level resulting from the design storm event, to the building platform level or **underside** of the floor joists or structural concrete slab of the building.

For open channels and streams, a minimum freeboard of 500 mm shall be adopted for the primary level of protection flow.

Vehicle bridges must have a freeboard of 600 mm to the underside of the bridge structure, or 1200 mm where there is a possibility of large trees in the waterway.

2. I also wish to note that we may be building over the current large ovoid stormwater main as shown below. Can you confirm in principle that we can do so, pending agreements on how to protect the main? Thanks



Regards

Graham Nash BSc(Dist) IEng MICE Associate, Land Infrastructure, Aurecon T +64 4 4718768 M +64 27 2516079 Graham.Nash@aurecongroup.com

#### DISCLAIMER

From: Graham Nash
Sent: Tuesday, 7 September 2021 5:08 pm
To: Sarah Zhou <<u>Sarah.Zhou@wellingtonwater.co.nz</u>>
Cc: Modelling Team <<u>Modelling.Team@wellingtonwater.co.nz</u>>; Ahmed Hassan
<<u>Ahmed.Hassan@wellingtonwater.co.nz</u>>
Subject: RE: MFC - Car Park Flood Sources and Levels-wwl - 7 Sep 2021

#### Hi Sarah,

Our connection point for wastewater would be in Wakefield St around Opera House Lane or east of (depending upon if we can make a direct connection or have to buffer storage on site prior to release). Same for our water supply, rather than a connection from Jervois

#### Thanks



Graham Nash BSc(Dist) IEng MICE Associate, Land Infrastructure, Aurecon T +64 4 4718768 M +64 27 2516079 Graham.Nash@aurecongroup.com

#### DISCLAIMER

From: Sarah Zhou <<u>Sarah.Zhou@wellingtonwater.co.nz</u>>
Sent: Tuesday, 7 September 2021 4:57 pm
To: Graham Nash <<u>Graham.Nash@aurecongroup.com</u>>
Cc: Modelling Team <<u>Modelling.Team@wellingtonwater.co.nz</u>>; Ahmed Hassan
<<u>Ahmed.Hassan@wellingtonwater.co.nz</u>>
Subject: RE: MFC - Car Park Flood Sources and Levels-wwl - 7 Sep 2021

#### Hi Graham

Ahmed in our modelling team is currently assessing the wastewater capacity. I understand you don't have a scheme plan at the moment.

Is it possible for you to identify the likely wastewater connection point? This will make wastewater assessment easier and faster.

Please find the water supply info below:

- The model shows that minimum pressure at the point of supply (assuming to be on the Wakefield Street) on the existing 200mm public main is expected to be about 75-80m, which meets the level of service criteria for pressure. The model also indicates that available fire flow capacity from the existing hydrants is expected to be compliant with their requirement of FW2. This assumption (FW2) needs to be confirmed by a fire engineer by assessing the actual fire risk for the proposed building.
- The proposed connection is likely to be OK if it is made to the 250mm pipe (the pipe crossed Wakefield Street) or to the 200mm on the other side of the road. Please provide design report, including all calculation and assumptions to ensure connection pipe is sized appropriately.

This modelling assessment only represents the existing network based on WWL hydraulic model developed in 2018. The analysis takes no account of developments that have occurred since then,

currently underway, or future developments. Non-hydraulic parameters like pipe age, conditions and likelihood of their failure have not been assessed. Please also note the above are just the result of WWL hydraulic model which could be impacted by day-to-day operational changes within the network and may need to be verified in the field through pressure logging and hydrant flow tests.



Thanks Sarah

Sarah Zhou Senior Engineer - Land Development Wellington Tel +64 4 912 4531 Mob 021 309 998 Private Bag 39804, Wellington Mail Centre 5045 Level 4, IBM House, 25 Victoria Street, Petone, Lower Hutt WWW.Wellingtonwater.co.nz

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From: Modelling Team <<u>Modelling.Team@wellingtonwater.co.nz</u>>
Sent: Tuesday, September 7, 2021 10:32 AM
To: Sarah Zhou <<u>Sarah.Zhou@wellingtonwater.co.nz</u>>
Subject: RE: MFC - Car Park Flood Sources and Levels-wwl - Urgent

Hi Sarah,

There are at least three different connections to the local network over this area, can you point roughly to the location of the proposed building? Or I will have to assess every possible connection which will take a while.

Ngā mihi Ahmed

Ahmed Hassan Senior Hydraulic Modelling – Land Development



Tel 04 912 4400 Mob 021 354 764 Private Bag 39804, Wellington Mail Centre 5045 Level 4, IBM House, 25 Victoria Street, Petone, Lower Hutt www.wellingtonwater.co.nz

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From: Sarah Zhou <<u>Sarah.Zhou@wellingtonwater.co.nz</u>>
Sent: Friday, 3 September 2021 9:25 am
To: Iman Aghamohammadi <<u>Iman.Aghamohammadi@wellingtonwater.co.nz</u>>; Ahmed Hassan
<<u>Ahmed.Hassan@wellingtonwater.co.nz</u>>; Modelling Team <<u>Modelling.Team@wellingtonwater.co.nz</u>>
Subject: FW: MFC - Car Park Flood Sources and Levels-wwl - Urgent
Importance: High

(Water & wastewater) Hi Guys

Can you provide modelling info for Michael Fowler Centre car park site? Further development is proposed at this area. (specific location is not confirmed)
Please note the following:

Sewer:

The proposed building I am advised is 14,300sq.m over 7 levels. At 400pp/ha this is a PWWF of around 9l/s.

#### Water supply:

Site demand of around 32l/s for 2/3 potable supply and fire flows assuming FW2 classification.



Thanks Sarah

Sarah Zhou Senior Engineer - Land Development Wellington Tel +64 4 912 4531 Mob 021 309 998 Private Bag 39804, Wellington Mail Centre 5045 Level 4, IBM House, 25 Victoria Street, Petone, Lower Hutt

#### www.wellingtonwater.co.nz

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From: Graham Nash <<u>Graham.Nash@aurecongroup.com</u>>
Sent: Thursday, September 2, 2021 5:57 PM
To: Sarah Zhou <<u>Sarah.Zhou@wellingtonwater.co.nz</u>>
Cc: Joey Narvasa <<u>Joey.Narvasa@wellingtonwater.co.nz</u>>
Subject: RE: MFC - Car Park Flood Sources and Levels-wwl - 20 Aug 2021

Hi folks

Just following this one up please

Thanks

**Graham Nash** BSc(Dist) IEng MICE Associate, Land Infrastructure, Aurecon DISCLAIMER

From: Graham Nash
Sent: Thursday, 26 August 2021 12:43 pm
To: 'Sarah Zhou' <<u>Sarah.Zhou@wellingtonwater.co.nz</u>>
Cc: 'Joey Narvasa' <<u>Joey.Narvasa@wellingtonwater.co.nz</u>>
Subject: RE: MFC - Car Park Flood Sources and Levels-wwl - 20 Aug 2021

Hi Sarah/Joey,

Separate to the below this could I also ask/get some information on the network constraints in the vicinity of the Michael Fowler centre with respect to stormwater, wastewater and potable water supply please? I anticipate that a sewer connection will be the most challenging

#### Sewer:

The proposed building I am advised is 14,300sq.m over 7 levels. At 400pp/ha this is a PWWF of around 9l/s.

#### Water supply:

Site demand of around 32I/s for 2/3 potable supply and fire flows assuming FW2 classification.

#### Stormwater:

The site is essentially fully impervious with downstream connection to large main to outfall – I do not expect significant issues here.

Many thanks Graham

**Graham Nash** BSc(Dist) IEng MICE Associate, Land Infrastructure, Aurecon **T** +64 4 4718768 **M** +64 27 2516079 **Graham.Nash@aurecongroup.com** 

DISCLAIMER

From: Graham Nash
Sent: Thursday, 26 August 2021 12:13 pm
To: 'Sarah Zhou' <<u>Sarah.Zhou@wellingtonwater.co.nz</u>>
Cc: 'Joey Narvasa' <<u>Joey.Narvasa@wellingtonwater.co.nz</u>>
Subject: RE: MFC - Car Park Flood Sources and Levels-wwl - 20 Aug 2021

Hi folks,

Any update on this please? Even some loose indication would be good thanks. Thanks

**Graham Nash** BSc(Dist) IEng MICE Associate, Land Infrastructure, Aurecon **T** +64 4 4718768 **M** +64 27 2516079 **Graham.Nash@aurecongroup.com** 

DISCLAIMER

From: Graham Nash
Sent: Sunday, 22 August 2021 7:14 pm
To: Sarah Zhou <<u>Sarah.Zhou@wellingtonwater.co.nz</u>>
Cc: Joey Narvasa <<u>Joey.Narvasa@wellingtonwater.co.nz</u>>
Subject: RE: MFC - Car Park Flood Sources and Levels-wwl - 20 Aug 2021

Hi Sarah,

At this point it's not clear - best to assume coverage of existing thanks

Cheers

Graham Nash BSc(Dist) IEng MICE Associate, Land Infrastructure, Aurecon T +64 4 4718768 M +64 27 2516079 Graham.Nash@aurecongroup.com

DISCLAIMER

From: Sarah Zhou <<u>Sarah.Zhou@wellingtonwater.co.nz</u>>
Sent: Friday, 20 August 2021 9:11 am
To: Graham Nash <<u>Graham.Nash@aurecongroup.com</u>>
Cc: Joey Narvasa <<u>Joey.Narvasa@wellingtonwater.co.nz</u>>
Subject: RE: MFC - Car Park Flood Sources and Levels-wwl - 20 Aug 2021

Hi Graham

Do you have a rough location of interest? Or the whole car park area? Would you be able to roughly mark up the extent you are interested?

Thanks Sarah

Sarah Zhou Senior Engineer - Land Development

Wellington
Tel +64 4 912 4531 Mob 021 309 998
Private Bag 39804, Wellington Mail Centre 5045
Level 4, IBM House, 25 Victoria Street, Petone, Lower Hutt

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Wellington Water is owned by the Hutt, Porirua, Upper Hutt and Wellington city councils and Greater Wellington Regional Council. We manage their drinking water, wastewater and stormwater services.

From: Graham Nash <<u>Graham.Nash@aurecongroup.com</u>>
Sent: Thursday, August 19, 2021 5:28 PM
To: Joey Narvasa <<u>Joey.Narvasa@wellingtonwater.co.nz</u>>; Sarah Zhou <<u>Sarah.Zhou@wellingtonwater.co.nz</u>>;
Subject: MFC - Car Park Flood Sources and Levels

Hi Joey/Sarah,

We're looking at a site in the Michael Fowler Centre car park for development.

I see that the site as existing is subject to flooding of between 0.5-1m in 1:200+CC.

Can we have more detail on the flooding source please and FFLs – I assume the shapefile includes sea level rise/surge/increased rainfall intensity?

Our client will no doubt have to raise FFLs to above the flood levels + freeboard (0.3m or 0.5m per 4.2.9 of Regional Standard?).

What is not clear is if we'll need to provide compensation for infill if surface water related, or if the flooding is mainly sea level rise driven in which case compensatory storage is presumably not required, or to a lesser degree



Thanks, and happy to discuss

Graham Nash BSc(Dist) IEng MICE Associate, Land Infrastructure, Aurecon T +64 4 4718768 M +64 27 2516079 Graham.Nash@aurecongroup.com Spark Central, Level 8, 42-52 Willis Street, Wellington New Zealand 6011 PO Box 1591, Wellington 6140 aurecongroup.com





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#### **Brock Goodison**

From:	René van Lierop
Sent:	Wednesday, 29 September 2021 2:14 PM
То:	Nadia Nitsche (nadia.nitsche@wellingtonwater.co.nz); Alistair Osborne; Sarah Zhou; Graham Nash
Cc:	Marcus Welby
Subject:	RE: MFC Office Building - Meeting Wlg Water - 28 Sep 2021

Hi All

As discussed in our meeting, we have run the 10yr and 10yr+CC design events and the results are presented in the summary table below.

Please note:

- Peak Water Levels for the 100yr and 100yr+CC events have gone up slightly as the peak flood elevation
  occurs at the eastern end of the building (close to Taranaki St intersection) instead of the western end (near
  the MFC).
- The increase in peak flood levels in that area is nearly 30mm The increase for the 10yr events are 10mm.

We haven't been able to plot the difference maps yet.

Desian Event	Rainfall	Peak Tide Level	Computed Pea (in m W	Variance	
		(in m WVD-53)	Existing Development	Proposed Development	(in m)
100yr Existing Climate	100yr ARI	1.1m WVD-53	2.61	2.63	+0.02
100yr Incl Climate Change	100yr ARI + 20%	2.1m WVD-53	2.80	2.83	+0.03
10yr Existing Climate	10yr ARI	1.1m WVD-53	2.39	2.40	+0.01
10yr Incl Climate Change	10yr ARI + 20%	2.1m WVD-53	2.58	2.59	+0.01

#### Regards

René

René van Lierop Associate, Water, Aurecon M +64 021 740930 Rene.vanLierop@aurecongroup.com

DISCLAIMER

From: René van Lierop
Sent: Tuesday, 28 September 2021 2:57 pm
To: Nadia Nitsche <nadia.nitsche@wellingtonwater.co.nz>; Alistair Osborne
<Alistair.Osborne@wellingtonwater.co.nz>; Sarah Zhou <Sarah.Zhou@wellingtonwater.co.nz>; Graham Nash

#### Hi Team

Flood modelling results to date are shown below:

Design Event	Rainfall	Peak Tide Level	Computed Peak Flood Levels (in m WVD-53)		
		(in m WVD-53)	Existing Development	Proposed Development	
100yr Existing Climate	100yr ARI	1.1m WVD-53	2.57	2.59	
100yr Incl Climate Change	100yr ARI + 20%	2.1m WVD-53	2.74	2.76	

Current proposed Finished Floor Level (FFL) is 2.9m WVD-53. This would provide for 140mm freeboard.

Impact on flood levels is an increase of up to 20mm. Acceptance of this increase depends on current flood risk of buildings and usage (i.e. residential or commercial).

Actions from the meeting are:

- Aurecon to model ARI 10yr & 10yr+CC events
- Aurecon to provide flood difference maps to show the extent of the 20mm increase in peak flood levels.
- Aurecon to report results
- WIg Water to check and provide FFL's of buildings in the area
- Wig Water to confirm requirements regarding freeboard
- Wig Water to confirm acceptance of difference in flood levels following Aurecon report on additional model runs

Regards

René

René van Lierop Associate, Water, Aurecon M +64 021 740930 www.linkedin.com/in/Rene-van-Lierop Rene.vanLierop@aurecongroup.com Spark Central, Level 8, 42-52 Willis Street, Wellington New Zealand 6011 PO Box 1591, Wellington 6140 aurecongroup.com



Whakahāngā schakaaro Kia māia, kia kaha, mahi tahi



Appendix C Existing Services Summary











WCC Stormwater	DN375 concrete	Runs along northeast side of existing temporary building within the site.
	main	
	DN225, transitioning to a DN300 concrete main	Runs along the southeast side of the existing temporary building.
	Ovoid 2210mm by 1220mm main	Runs through site to the west of the temporary building

WCC Wastewater	DN350 CI pressure main	Running along Wakefield St carriageway, approximately 5m offset from the site boundary.
	DN250 PE100 main	Running along Wakefield St carriageway, approximately 7m offset from the site boundary
	DN450 concrete discharge pipe	Running along Wakefield ST carriageway and footpath with kink in alignment, with offset varying from approximately 2m to 11m from site boundary
	DN150 PE100 main	Running along Wakefield St carriageway, approximately 9m offset from the site boundary
	Pumpstation, with DN315 PE100 connecting main	East corner of site near MFC building.





Appendix D Impervious Area Assessment



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## COMPARISON

drawing no.

#### project name MFC Car Park Development

## Appendix E CCTV Investigation Reports

## EMERGENCY WASTE CHAMBER INSPECTION



GENERAL - HIGH PRESSURE WATER JETTING CONDUCTED TO CREATE ACCESS TO PERIMETERS (FLOOR TO WALL AND WALL TO CEILING) AND BOTH SIDES OF CENTRAL COLUMNS, HIGH AND LOW LEVEL. ALL WALLS WERE GIVEN A CLEAN TO ENSURE GOOD VIDEO FOOTAGE. LOCALISED HAMMER TESTING CONDUCTED TO AREAS OF QUESTION. NONE OF THESE AREAS PRESENTED OBVIOUS DEFECTS OR DRUMMINESS. OVERALL CONDITION OF CHAMBER APPEARED SOUND.



	Section	on Insp	Dection - 2/12	/2021 - WCC_	SWP025	801		
Section No. 1	Section No. Asset No. 1 WCC_SWP025801		Contract Number CC8018	Date 2/12/2021	Time 9:10 an	n	Weath	er
WELLINGTON	Name o S Jo	of Coder ohns	Operator <b>KF</b>	L.T McGuiness SW MFC 1		10.	Clien	t
Pipe Length	37.21 m		Material	Cast In-Situ concrete or Mortar	Use		Stormwate	r
Setup	Upstream	node	Snape	Arch shaped	Ріре Туре	C.	sravity	
Surveyed Length	37.21 m		Dia/Height	0 mm	Currency	C	Surrent Insp	ection
Joint Spacing	0.00 m		Width	2,210 mm	Inspection	I	nspection co	omplete
U/S Node	WCC_SW	024022		D/S Node	WCC_SW0242	86		
U/S Node Type	Stormwate	er Manhole		D/S Node Type	Stormwater Ma	anhole		
U/S Pipe Depth				D/S Pipe Depth				
U/S Location	170 WAKE	FIELD STR	EET	D/S Location				
General Comment								
1:227	m +	oc	Observation			MPEG	Photo	Score
WCC_SW024022	0.00	IS	Inspection start, node re MH WCC_SW024022	ference: WCC_SW0240	22 / FROM US	00:00:00		
	0.00	OPS	Obstruction permanent, diameter from 11 o'clock	reduction of up to 10% o to 1 o'clock / WOODEN	f the pipe BEAM?	00:00:38	WCC_SW P025801_f 0027fa6-8	10/15
	0.00	LPS	Lateral protruding, into the pipe at 11 o'clock	ne pipe by 10% or less o	f the diameter	00:00:45		10
	0.00	LFXS	Lateral sealing faulty, sm main pipe which are not	nall gaps between the lat open at 11 o'clock / 225	eral and the mm?	00:00:45	WCC_SW P025801_ 9e7f63d7-	1
	0.00	LFXS	Lateral sealing faulty, sm main pipe which are not	nall gaps between the lat open at 11 o'clock / 225	eral and the mm?	00:00:51	WCC_SW P025801_ acd7d454-	1
	0.00 S01	DES	Debris silt, small, clear d start / STONES TO 37.2	iameter reduced by less 1	than 10%,	00:01:07		
	0.58	LXR	Lateral problem, roots se JFXL, RI S	een inside the lateral at 1	0 o'clock /	00:01:49	WCC_SW P025801_ 32fd10de-	
	0.58	LFXS	Lateral sealing faulty, sm main pipe which are not	nall gaps between the lat open at 10 o'clock / 525i	eral and the mm?	00:01:49		1
	8.89	SRCS	Wall loss due to corrosic is corroded) at 11 o'clocl	n exposing reinforcemer	nt steel (which	00:03:42	WCC_SW P025801_ 23c88c88-	20 / 15
	11.04	SAES	Concrete aggregate is vi the pipe circumference f	sible, damage covering rom 11 o'clock to 12 o'clo	up to 10% of ock	00:04:17		6/5
	11.05	LFXS	Lateral sealing faulty, sm main pipe which are not	nall gaps between the lat open at 9 o'clock / 225m	eral and the m?	00:04:18		1
	18.26	RIFS	Fine roots, roots restrict o'clock to 11 o'clock	flow by 10% or less of fu	ll flow from 10	00:05:15		5
	19.30	RIFS	Fine roots, roots restrict o'clock to 11 o'clock	flow by 10% or less of fu	ll flow from 10	00:05:17		5
	21.00 S02	RIFS	Fine roots, roots restrict o'clock to 2 o'clock / RI S	flow by 10% or less of fu S TO 35.68	ll flow from 10	00:06:08		
	25.99	SAES	Concrete aggregate is vi the pipe circumference f	sible, damage covering rom 12 o'clock to 1 o'cloc	up to 10% of ck	00:07:01		6/5
	27.40	LFXS	Lateral sealing faulty, sm main pipe which are not	nall gaps between the lat open at 9 o'clock / 300m	eral and the m?	00:07:26		1
	27.40	OPS	Obstruction permanent, diameter from 11 o'clock CONCRET SLAB?	reduction of up to 10% o to 1 o'clock / REINFOR	f the pipe CED	00:07:27	WCC_SW P025801_ b67022bc-	10 / 15



## Section Inspection - 2/12/2021 - WCC\_SWP025801

					•••••	•••		
Section No. <b>1</b>	Asset No. WCC_SWP025801		Asset No.         Contract Number         Date         Time           WCC_SWP025801         CC8018         2/12/2021         9:10 a		Time <b>9:10 an</b>	ime We <b>0 am</b>		er
Town or Suburb WELLINGTON	Name S .	e of Coder Johns	Operator <b>KF</b>	Video Reference L.T McGuiness SW MFC 1	Drawing N	No. Client		t
1:227	m +	oc	Observation			MPEG	Photo	Score
	32.51	IPL	Infiltration, gushing or jet o'clock / GUSHING FLO	ting (pressure flow) from W	5 o'clock to 7	00:09:06	WCC_SW P025801_ 9f3da986-	30 / 30
	35.68 F02	2 RIFS	Fine roots, roots restrict o'clock to 2 o'clock / RI S	flow by 10% or less of ful 5 TO 35.68	s of full flow from 10 00:06:0			73
	37.21 F01	1 DES	Debris silt, small, clear d / STONES TO 37.21	iameter reduced by less	than 10%, end	00:01:07		558
WCC_SW024286	37.21	IE	Inspection ends, node re MH WCC_SW024286	ference: WCC_SW0242	86 / AT DS	00:11:24		

			2.100						3
11	30	2.34	87.00	4	11	50	19.79	736.55	5
STR No. Def	STR Peak	STR Mean	STR Total	STR Grade	SER No. Def	SER Peak	SER Mean	SER Total	SER Grade



### Section Pictures - 2/12/2021 - WCC\_SWP025801

Town or Suburb WELLINGTON Location

Date 2/12/2021 Asset No. WCC SWP025801 Section No. 1



WCC\_SWP025801\_f0027fa6-81b5-448b-bfd8-8a4cafeb513b\_ 20211203\_110113\_319.jpg, 00:00:38, 0.00m Obstruction permanent, reduction of up to 10% of the pipe diameter from 11 o'clock to 1 o'clock / WOODEN BEAM?



WCC\_SWP025801\_9e7f63d7-ed53-4318-8822-9b06ced52c9 8\_20211203\_110148\_592.jpg, 00:00:45, 0.00m Lateral sealing faulty, small gaps between the lateral and the main pipe which are not open at 11 o'clock / 225mm?



WCC\_SWP025801\_acd7d454-96b1-4f18-9d38-405470fc8cdd \_20211203\_110245\_257.jpg, 00:00:51, 0.00m Lateral sealing faulty, small gaps between the lateral and the main pipe which are not open at 11 o'clock / 225mm?



WCC\_SWP025801\_32fd10de-402a-41a3-a977-52e1d9ab818 3\_20211203\_110617\_934.jpg, 00:01:49, 0.58m Lateral problem, roots seen inside the lateral at 10 o'clock / JFXL, RI S



### Section Pictures - 2/12/2021 - WCC\_SWP025801

Town or Suburb WELLINGTON Location

Date 2/12/2021 Asset No. WCC SWP025801 Section No. 1



WCC\_SWP025801\_23c88c88-ea71-4790-8a0f-c1ef42a8b44f \_20211203\_111619\_725.jpg, 00:03:42, 8.89m Wall loss due to corrosion exposing reinforcement steel (which is corroded) at 11 o'clock



WCC\_SWP025801\_e20a2ae7-e334-402f-9010-ee3999ad05a a\_20211203\_111620\_055.jpg, 00:03:42, 8.89m Wall loss due to corrosion exposing reinforcement steel (which is corroded) at 11 o'clock



WCC\_SWP025801\_b67022bc-c7a1-4f64-981d-72f80bee0da8 \_20211203\_115723\_284.jpg, 00:07:27, 27.40m Obstruction permanent, reduction of up to 10% of the pipe diameter from 11 o'clock to 1 o'clock / REINFORCED CONCRET SLAB?



WCC\_SWP025801\_4da97f73-fc7c-437a-8d5f-8b43f2c9b4ac\_ 20211203\_115802\_728.jpg, 00:07:27, 27.40m Obstruction permanent, reduction of up to 10% of the pipe diameter from 11 o'clock to 1 o'clock / REINFORCED CONCRET SLAB?



### Section Pictures - 2/12/2021 - WCC\_SWP025801

Town or Suburb

Location

Date 2/12/2021 Asset No. WCC\_SWP025801

Section No.

1



WCC\_SWP025801\_9f3da986-010a-4a12-aeea-08f05809520b \_20211203\_120615\_827.jpg, 00:09:06, 32.51m Infiltration, gushing or jetting (pressure flow) from 5 o'clock to 7 o'clock / GUSHING FLOW

		Se	ctio	on In	spe	ectio	n - 2	/12/	2021 - \	NCC <sup>_</sup>	SV	VP025	804			
Section No. 2		Asset No. WCC_SWP025804		Contract Number5804CC8018		er	Date 2/12/2021			Time 9:39 an	n	Weather				
Town or Suburb		Name of Coder				Operator			Video Ref	Drawing No.			Client			
	BTON		S Jonns			KF L.1 McGuiness SW MFG				SW WIFC 2						
Pipe Length 9			9.24 m			Material			Steel Reinforced Concrete			Use		Stormwater		
Setup		Opstream node		3	Dia/Hoight			Circular			Gravity		otion			
Surveyed Length		9.24 m				/ia/Heigi	nt		3/5 mm	Curr	ency		Inspection complet			
		WCC SW024287			V	viatri					wc		96	inspecti	on co	mpiere
U/S Node Type		Store	-SWC	or Manho					D/S Node Ty	00	Stor	mwator M	anholo			
U/S Rine Depth									D/S Ribe Der	pe oth	annoic					
U/S Fipe Deptin		170 WAKEFIELD STR			TRFF	REET			D/S Location							
General Commont		1701				- 1										
	nont															
1:70		m +		oc	O	bservati	ion						MPEG	Pho	to	Score
	0 4007															
	24287															
					Ind	enaction	start no	do rof	oronco: WCC	SIM0242	07/5	POMUS				
		0.00		IS	M	H WCC	_SW0242	287		_3110242	.07 / 1	KOW 03	00:00:00	)		
					_											
		0.00		DES	De	ebris silt FONES/	, small, c SII T	lear di	ameter reduc	ed by less	than	10% /	00:00:20	)		15
					01		OILT									
		1.43		JEDS	Jo	oint faulty	y, minor d	crackir	ng or spalling	at 1 o'cloc	:k / D/	AMAGE	00:01:25	5		3
		1.40		01 20	тс	D JOINT							00.01.20	•		0
		3 03		IEYI	Jo	oint faulty	y, major o	crackir	ng or spalling,	likely thro	ough t	he pipe	00.02.10	WCC_	SW	10
¥		5.92		WLC	Wa Cl	all from 7 ear wate S MH.	11 o'cloci er / WATI	k to 1 k	o'clock / SEAI VEL INCREA	LING RING		IBLE DEBRIS IN	00:03:15	135f07	7e1-	10
	/ _	6.78		JFXL	Jo wa	oint faulty all from 2	y, major o 11 o'clocl	crackir k to 1	ng or spalling, o'clock / SEAI	likely thro LING RINO	ough t G VIS	he pipe IBLE	00:03:28	3		10
		S01	DES	De sta	ebris silt art / SIL⁻	, small, c T/STONE	lear di S TO	ameter reduc 9.24	ed by less	s than	10%,	00:03:28	WCC_ 8 P0258	SW 04_		
		7.66		CCL	Cr tha o'c	rack circ at it exte clock / E	umferent ends thro D FROM	ial, cra ugh to I CRA	ack, large, op the outside w CK	en and the vall from 9	ere is o'cloo	evidence ck to 3	00:04:15	WCC_ 5 P0258 14e520	SW 04_ 0b2-	12
		7.66		EDS	Er 10	ncrustati )% from	on depos 9 o'clock	sits, clo to 3 c	ear diameter i o'clock / FROM	s reduced I/ CRACK	by le	ss than	00:04:15	5		10
		8.27		GC	General comment / EELS IN PIPELINE								00:04:43	WCC_ P0258 6295f	SW 04_ 24-	
		9.24	F01	DES	De / S	Debris silt, small, clear diameter reduced by less than 10%, end / SILT/STONES TO 9.24							00:03:28	3		36
	24286	9.24		IE	Ins	spection	ends, no	ode re	ference: WCC	C_SW0242	286 /	AT DS	00:06:54	Ļ		
STR No. Def	STR Pe	ak	STR	Mean	STR	Total	_5770242 STR Gr	∠ŏb ade ∣	SER No. Def	SER P	eak	SER Mea	an SEI	R Total	SEF	R Grade
4	12		3	79	35	5.00	5		3	25		6.59	6	0.85		5

LT McGuiness CC8018 Wakefield St



### Section Pictures - 2/12/2021 - WCC\_SWP025804

Town or Suburb WELLINGTON Location

Date 2/12/2021

Asset No. WCC SWP025804 Section No. 2



WCC\_SWP025804\_135f07e1-ae30-49f0-9126-406532f61802 \_20211203\_133540\_002.jpg, 00:02:19, 3.93m Joint faulty, major cracking or spalling, likely through the pipe wall from 11 o'clock to 1 o'clock / SEALING RING VISIBLE



WCC\_SWP025804\_96b1aac9-a355-4d8b-b2f9-7a4c358eabe 5\_20211203\_134037\_980.jpg, 00:03:28, 6.85m Debris silt, small, clear diameter reduced by less than 10%, start / SILT/STONES TO 9.24



WCC\_SWP025804\_14e520b2-e007-4add-bc6b-7ee710cc777 6\_20211203\_134315\_289.jpg, 00:04:15, 7.66m Crack circumferential, crack, large, open and there is evidence that it extends through to the outside wall from 9 o'clock to 3 o'clock / ED FROM CRACK



WCC\_SWP025804\_6295fca4-a9ba-413b-936a-9f7bf3569196 \_20211203\_134529\_352.jpg, 00:04:43, 8.27m General comment / EELS IN PIPELINE


# WinCan

### Section Pictures - 2/12/2021 - WCC\_SWP025804

Town or Suburb WELLINGTON Location

Date 2/12/2021

Asset No. WCC\_SWP025804 Section No.

2



WCC\_SWP025804\_29ddb046-8a51-4c67-bcf9-0f6c76bcba06 \_20211203\_134605\_062.jpg, 00:04:43, 8.27m General comment / EELS IN PIPELINE

## Appendix F Wakefield St Thresholds Survey (Spencer Holmes)





MB CI

RESOL

Level 10 - 57 Willis Street PO Box 588 Wellington New Zealand (04) 472-2261 www.spencerholmes.co.nz

If the specific position or height of any wall, building or structure is critical to the design then its accuracy must be confirmed by SHL before it can be relied upon.



### Document prepared by

### **Aurecon New Zealand Limited**

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